



湖南大學  
HUNAN UNIVERSITY



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5TH INTERNATIONAL BRIDGE SEISMIC WORKSHOP (5IBSW)

# Time-domain dimension-reduction simulation for non-stationary stochastic ground motion

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- 1. Research Background**
- 2. Theoretical Basis**
- 3. DR representations in time domain**
- 4. Numerical examples**
- 5. Conclusion**

## ➤ Bridge infrastructures under stochastic dynamic actions



- ◆ Earthquakes, wind, and ocean waves are highly **random** in location and timing and may cause severe **damage** to civil engineering structures within a short period.
- ◆ Bridge failure lead to substantial casualties and economic losses, highlighting the need for **accurate stochastic modeling**.
- ◆ Measured records of disaster stochastic dynamic actions are **scarce**, while the **demands** for structural analysis, reliability assessment and uncertainty quantification continue to grow.
- ◆ Therefore, **artificial simulation** based on stochastic process modeling has become a commonly adopted approach.



Foundation for Reliability Assessment

**Accurate modeling for  
stochastic dynamic actions**

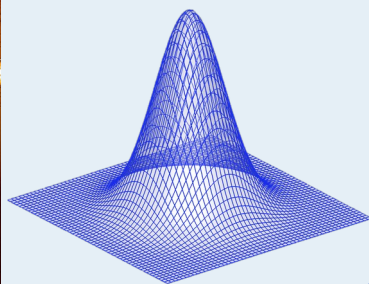
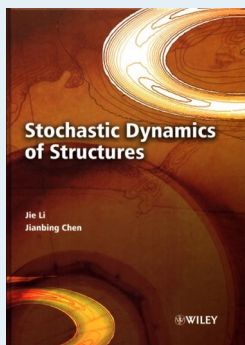
# Theoretic Background

## Transfer of randomness

### Theoretical Basis

#### Probability Density Evolution Method

- PDEM is a core foundation of the **third-generation structural design theory**.
- Stochastic dynamic **action modeling** provides an important basis for engineering stochastic dynamics.



### Input

#### Stochastic dynamic actions



Earthquakes



Wind



Ocean waves

...

### System

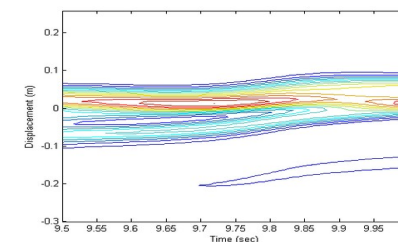
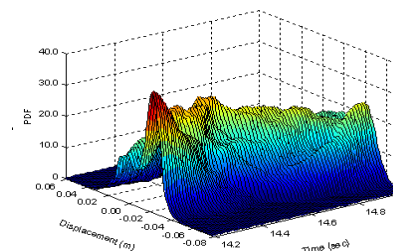
#### Bridge structures



Long-span bridges

### Output

#### Stochastic dynamic



Probability distribution  
Reliability assessment

**Accurate** stochastic action modeling is the **basis** for assessing the disaster-resilient reliability of bridge structures

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# Theoretical Basis

## Time-domain representations for One-dimensional univariate stochastic processes (1D-1V)

Fourier-Stieltjes integral form

$$\tilde{X}(t) = \int_{-\infty}^{\infty} A(t, \omega) e^{i\omega t} dZ(\omega) = \int_{-\infty}^{\infty} A(t, \omega) e^{i\omega t} H(\omega) dU(\omega)$$

Complex orthogonal  
increment processes

Time-frequency  
modulation function

Its increments satisfy

$$\begin{aligned} E[dZ(\omega)] &= 0, \quad E[dU(\omega)] = 0 & dZ(-\omega) &= dZ^*(\omega), \quad dU(-\omega) = dU^*(\omega) \\ E[dU(\omega)dU^*(\omega')] &= \frac{1}{2\pi} \delta_{\omega\omega'} d\omega & E[dZ(\omega)dZ^*(\omega')] &= \frac{1}{2\pi} S_X(\omega) \delta_{\omega\omega'} d\omega \end{aligned}$$

### 1 Stationary stochastic process

Auto-PSD

$$S_X(\omega) = |H(\omega)|^2$$

Frequency response function

Satisfying

$$H(-\omega) = H^*(\omega)$$

Time-domain representation

$$X(t) = \int_{-\infty}^{\infty} h(t - \tau) \int_{-\infty}^{\infty} e^{i\omega\tau} dU(\omega) d\tau = \int_{-\infty}^{\infty} h(t - \tau) dW(\tau) = \int_{-\infty}^{\infty} h(t - \tau) w(\tau) d\tau$$

Where  $w(\tau)$  satisfies

$$S_w(\omega) = 1$$

$$E[dw(\tau)] = 0, \quad E[dw(\tau)dw^*(\tau')] = \delta(\tau - \tau') d\tau$$

Fourier transform pairs

$$\begin{cases} H(\omega) = \int_{-\infty}^{\infty} h(\tau) e^{-i\omega\tau} d\tau \\ h(\tau) = \frac{1}{2\pi} \int_{-\infty}^{\infty} H(\omega) e^{i\omega\tau} d\omega \end{cases}$$

### 2 Non-stationary

EPSP

$$S_{\tilde{X}}(t, \omega) = |A(t, \omega)|^2 S_X(\omega) = |\tilde{H}(t, \omega)|^2$$

Fourier transform pairs

$$\tilde{H}(t, -\omega) = H^*(t, \omega)$$

$$\begin{cases} \tilde{H}(t, \omega) = \int_{-\infty}^{\infty} \tilde{h}(t, \tau) e^{-i\omega\tau} d\tau \\ \tilde{h}(t, \tau) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \tilde{H}(t, \omega) e^{i\omega\tau} d\omega \end{cases}$$

Time-domain representation

$$\tilde{h}(t, \tau) = h^*(t, \tau)$$

$$\begin{aligned} \tilde{X}(t) &= \int_{-\infty}^{\infty} \tilde{h}(t, t - \tau) \int_{-\infty}^{\infty} e^{i\omega\tau} dU(\omega) d\tau \\ &= \int_{-\infty}^{\infty} \tilde{h}(t, t - \tau) dW(\tau) \\ &= \int_{-\infty}^{\infty} \tilde{h}(t, t - \tau) w(\tau) d\tau \end{aligned}$$

Where  $dW(\tau)$  satisfies

$$E[dW(\tau)] = 0, \quad E[dW(\tau)dW^*(\tau')] = \delta_{\tau\tau'} d\tau$$

- ★ 1. Stochastic processes can be regarded as the result of **linear transformations** driven by linear time-varying or time-invariant systems.
- ★ 2. Time-domain analysis methods are **as crucial as** frequency-domain approaches. Their discretizations give rise to two typical simulation methods: the SRM in the frequency domain **and the linear filter method** in time domain.

# Theoretical Basis

## 1 Piecewise integral

$$X(t) = \int_{-\infty}^t h(t-\tau) dW(\tau) = \int_0^t h(t-\tau) dW(\tau) = \sum_{i=1}^n \int_{t_{i-1}}^{t_i} h(t-\tau) dW(\tau) + \int_n^t h(t-\tau) dW(\tau)$$

Approximation

$$X(t) \approx \sum_{i=1}^n h(t-t_i) \int_{t_{i-1}}^{t_i} dW(\tau) \quad t_i = i \times \Delta t, \quad i=0,1,\dots,N \quad t \in [t_n, t_{n+1}), \quad n=1,2,\dots,N$$

Constant

Omitted

## 2 Orthogonal random variables

$$W_i = \int_{t_{i-1}}^{t_i} dW(\tau)$$



$$V_i = \frac{W_i}{\sqrt{\Delta t}}$$

Satisfying

$$E[W_i] = 0, \quad E[W_i W_j] = \Delta t \delta_{ij}$$

$$E[V_i] = 0, \quad E[V_i V_j] = \delta_{ij}$$

- ★ 1. The time-domain representations for stochastic processes can be formulated as a linear combination of **standard orthogonal random variables** modulated by **deterministic functions**.
- ★ 2. Random variables is a set of **standard orthogonal random variables** whose probability distributions are not predetermined. This flexibility exactly opens possibilities for alternative representations of the orthogonal random variable set.

## 3 Discrete time-domain

A) **Stationary case**      B) **Non-stationary case**

$$X(t) \approx \sqrt{\Delta t} \sum_{i=1}^n h(t-t_i) V_i$$

$$\tilde{X}(t) \approx \sqrt{\Delta t} \sum_{i=1}^n \tilde{h}(t, t-t_i) V_i$$



standardized form



$$X^s(t) \approx \sigma_X \sum_{i=1}^n u_i(t) V_i$$

$$\tilde{X}^s(t) \approx \sigma_{\tilde{X}} \sum_{i=1}^n \tilde{u}_i(t) V_i$$

where

$$u_i(t) = \frac{h(t-t_i)}{\sqrt{\sum_{j=1}^n h^2(t-t_j)}}$$

$$\tilde{u}_i(t) = \frac{\tilde{h}(t, t-t_i)}{\sqrt{\sum_{j=1}^n \tilde{h}^2(t, t-t_j)}}$$

Convolution commutativity

$$X(t) \approx \sqrt{\Delta t} \sum_{i=0}^n h(t_i) V_{n-i}$$

$$\tilde{X}(t) \approx \sqrt{\Delta t} \sum_{i=0}^n \tilde{h}(t, t_i) V_{n-i}$$

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# DR representations in time domain

Construct orthogonal random variables from elementary random variables:

$$E[V_i] = 0, \quad E[V_i V_j] = \delta_{ij}$$

## 1 DR representations based on random orthogonal functions

a. Legendre orthogonal polynomial functions

Legendre polynomial function

$$\bar{V}_k = \sqrt{2k+1} P_k(\theta), \quad k=1, 2, \dots, N$$

Elementary random variable

Uniform distribution  $\theta \in (-1, 1)$

$$\begin{cases} P_{k+1}(x) = \frac{2k+1}{k+1} x P_k(x) - \frac{k}{k+1} P_{k-1}(x) \\ P_0(x) = 1, \quad P_1(x) = x \end{cases}$$

b. Hartley orthogonal basis

$$\bar{V}_k = \sqrt{2} \sin(k\theta + \alpha), \quad k=1, 2, \dots, N$$

Elementary random variable

Uniform distribution  $\theta \in (0, 2\pi)$

Inverse transform method

Gaussian distribution

$$\begin{aligned} \bar{V}_k &= \Phi^{-1} \left[ \frac{1}{2} + \frac{1}{\pi} \arcsin \sin(k\theta + \alpha) \right] \\ \Phi(x) &= \frac{1}{\sqrt{2\pi}} \int_{-\infty}^x \exp\left(-\frac{t^2}{2}\right) dt \end{aligned}$$

c. Trigonometric orthogonal basis

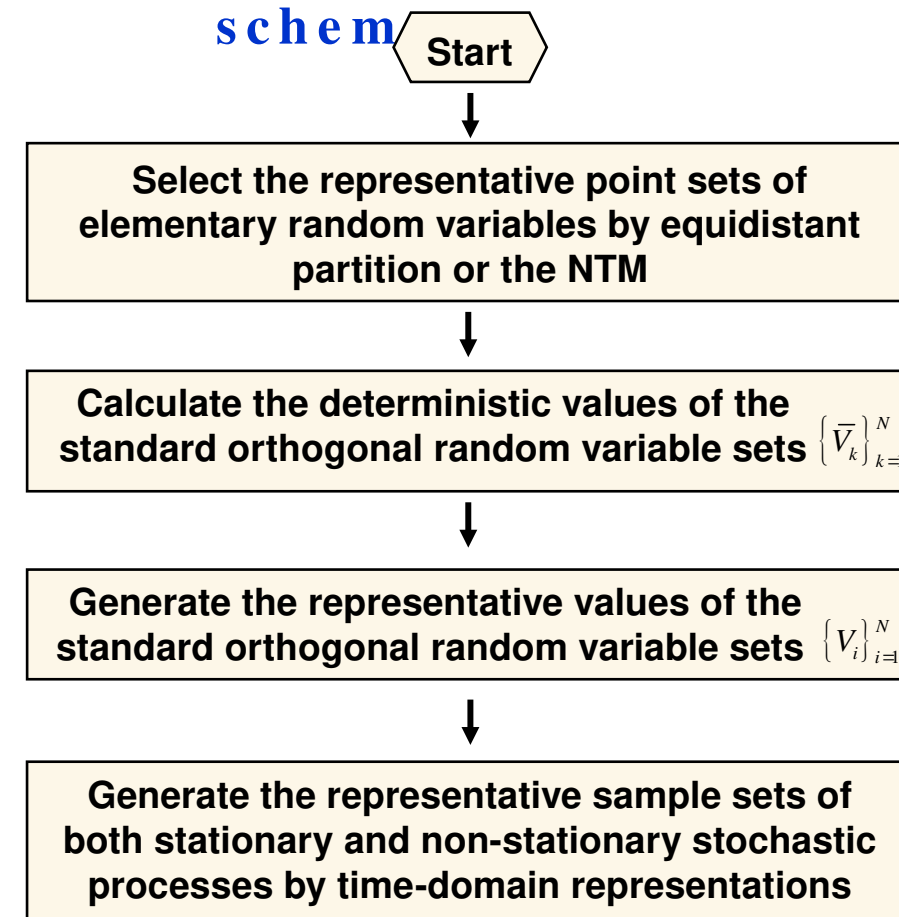
$$\bar{V}_{2k-1} = \sqrt{2} \sin(k\theta_1 + \alpha), \quad \bar{V}_{2k} = \sqrt{2} \cos(k\theta_2 + \alpha), \quad k=1, 2, \dots, N/2$$

Elementary random variable  $\theta_1, \theta_2 \in (0, 2\pi)$

Deterministic one-to-one mapping relationship

$$\{\bar{V}_k\}_{k=1}^N \rightarrow \{V_i\}_{i=1}^N$$

## 2 Implementation scheme



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# Numerical examples

## ◆ Simulation of stationary ground motion processes

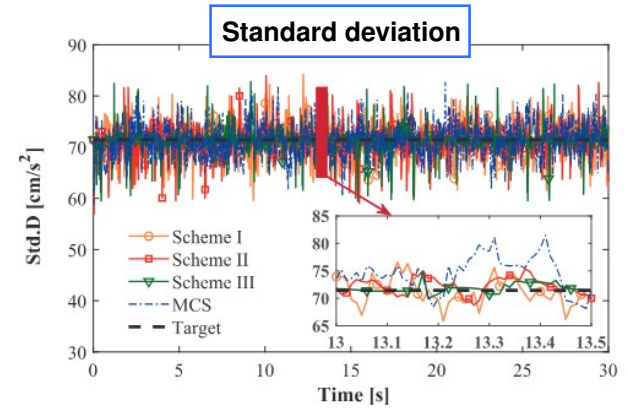
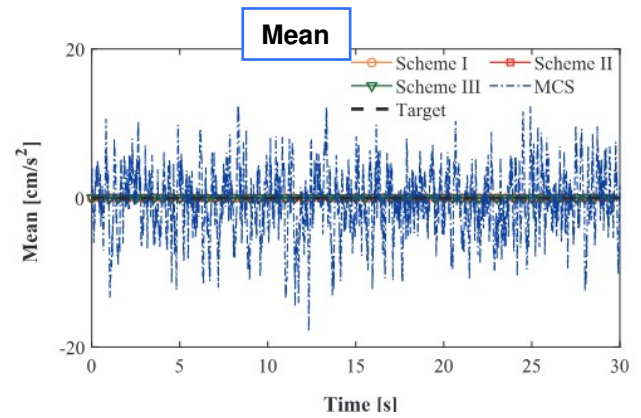
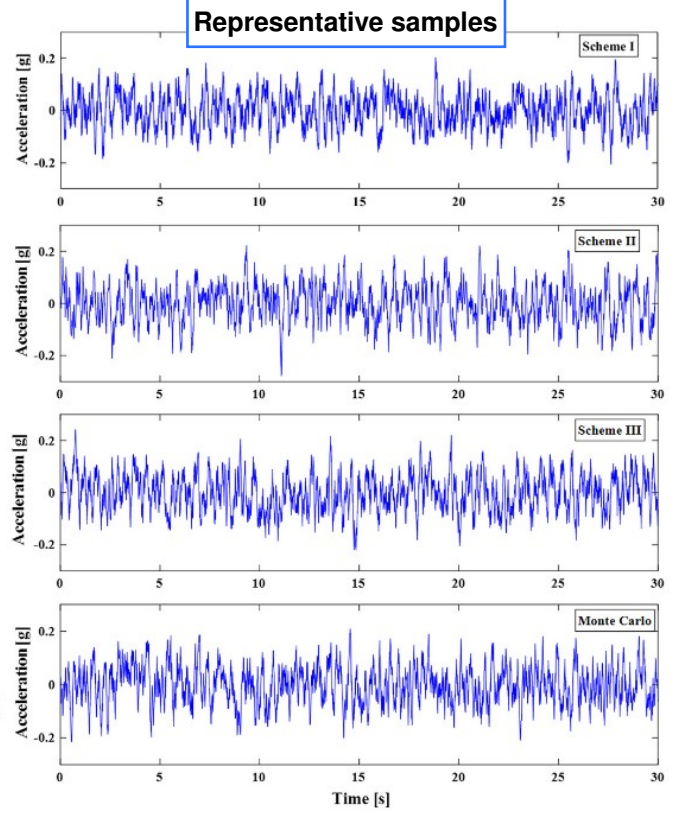
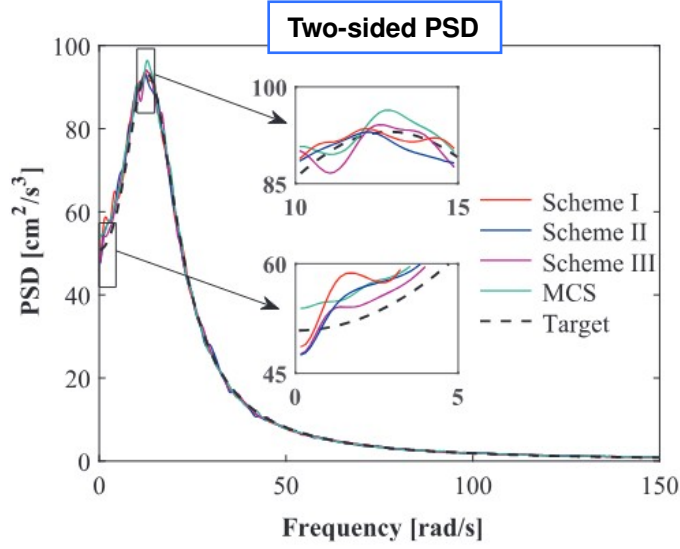
### Calculation parameters for simulation of stationary ground motions

| Parameters                  | Values                            | Parameters         | Values                               |
|-----------------------------|-----------------------------------|--------------------|--------------------------------------|
| Dominant frequency          | $\omega_g = 5\pi \text{ rad/s}$   | Damping ratio      | $\xi_g = 0.6$                        |
| PGA                         | $A_{\max} = 200 \text{ cm/s}^2$   | Peak factor        | $\gamma = 2.8$                       |
| The entire duration         | $T = 30 \text{ s}$                | Time step          | $\Delta t = 0.01 \text{ s}$          |
| The upper cut-off frequency | $\omega_u = 100\pi \text{ rad/s}$ | Frequency interval | $\Delta\omega = 0.209 \text{ rad/s}$ |

The parameters are based on the values for **stiff soils**.

### Comparison of simulation accuracy and efficiency between DR and MCS

| Simulation method | Scheme | M-ARE (%)              | SD-ARE (%) | PSD-ARE (%) | Time (s) |
|-------------------|--------|------------------------|------------|-------------|----------|
| DR(233)           | I      | $3.98 \times 10^{-3}$  | 3.67       | 2.65        | 3.87     |
|                   | II     | $3.73 \times 10^{-4}$  | 2.57       | 2.46        | 3.89     |
|                   | III    | $9.60 \times 10^{-11}$ | 1.39       | 2.49        | 4.41     |
| MCS(233)          |        | 5.22                   | 3.51       | 2.76        | 4.40     |
| MCS(500)          |        | 3.74                   | 2.44       | 2.50        | 8.84     |

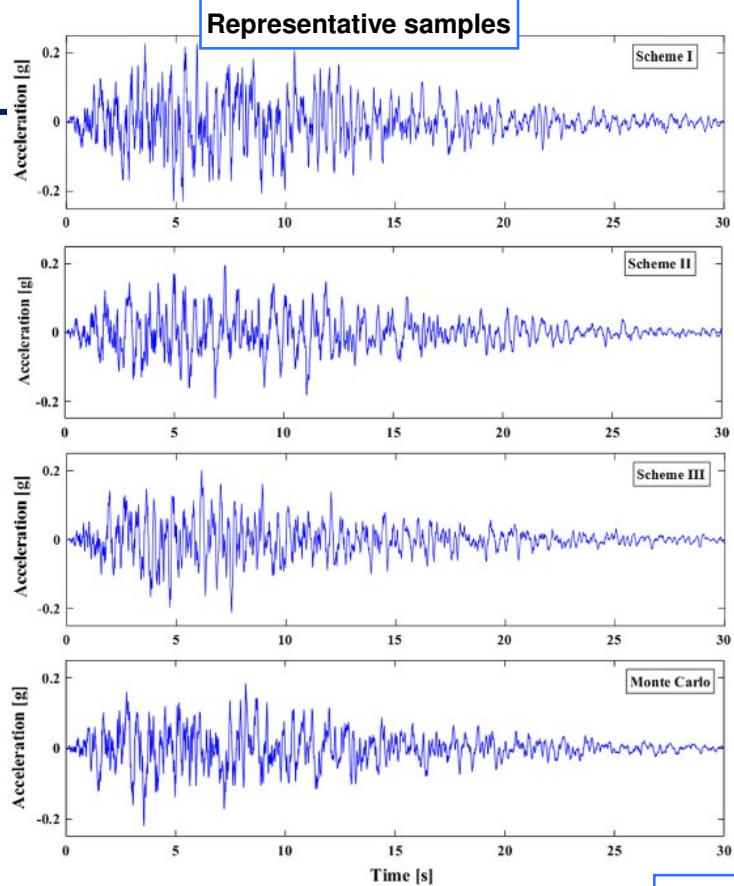
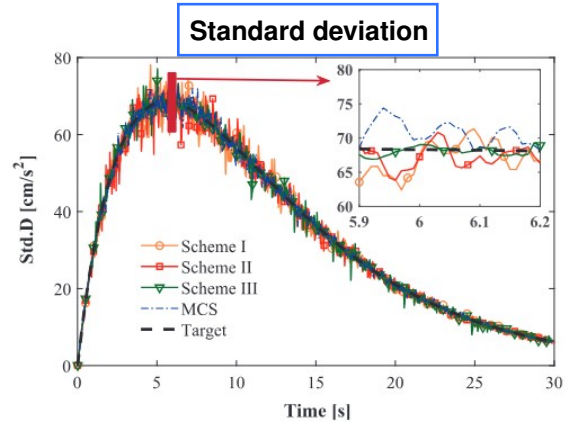
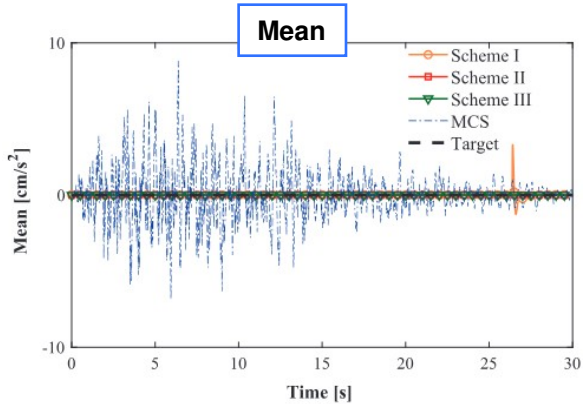
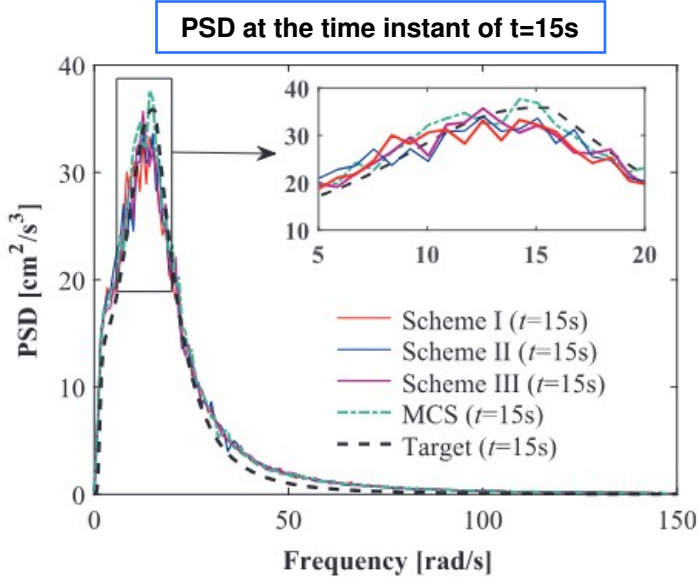


# Numerical examples

## ◆ Simulation of non-stationary ground motion processes

### Calculation parameters for simulation of non-stationary ground motions

| Parameters                                          | Values                            |
|-----------------------------------------------------|-----------------------------------|
| Dominant frequency of the rock bed                  | $\omega_f = 0.5\pi \text{ rad/s}$ |
| Damping ratio of the rock bed                       | $\xi_f = 0.6$                     |
| Parameter of the time-frequency modulation function | $a = 0.15 \text{ s}^{-1}$         |
| Number of representative samples                    | $n_{\text{sel}} = 233$            |





# Numerical examples

## ◆ Simulation of non-stationary ground motion processes

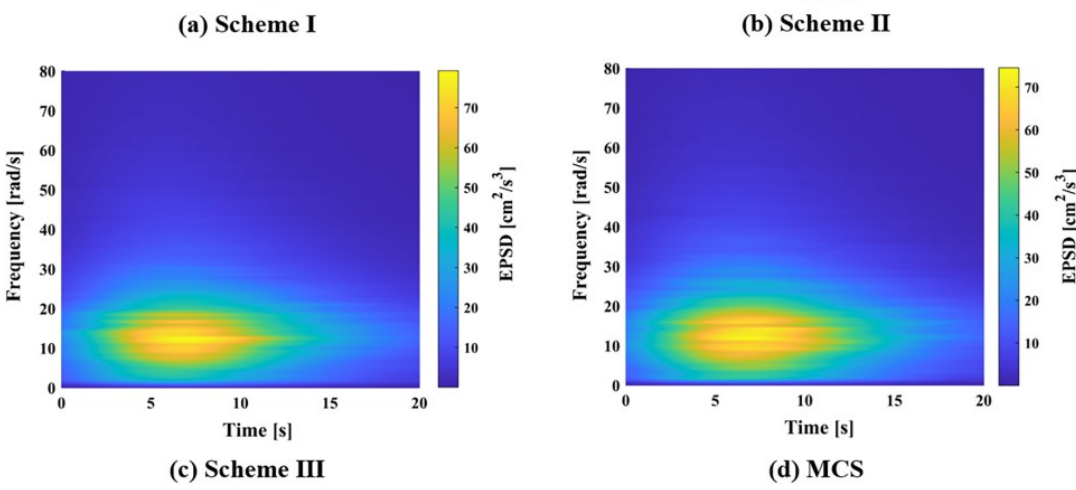
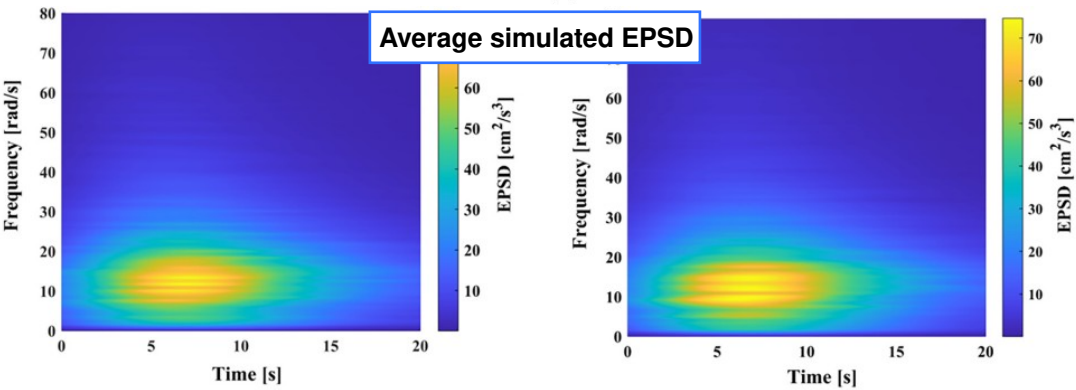
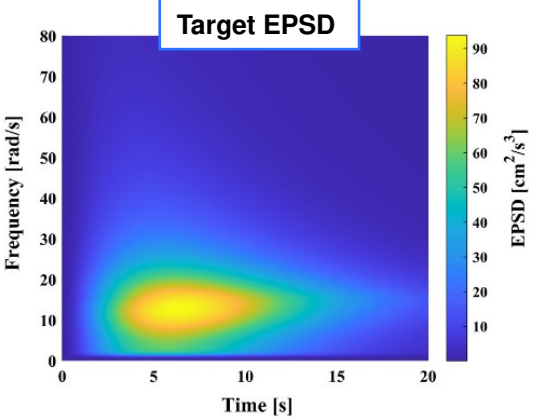
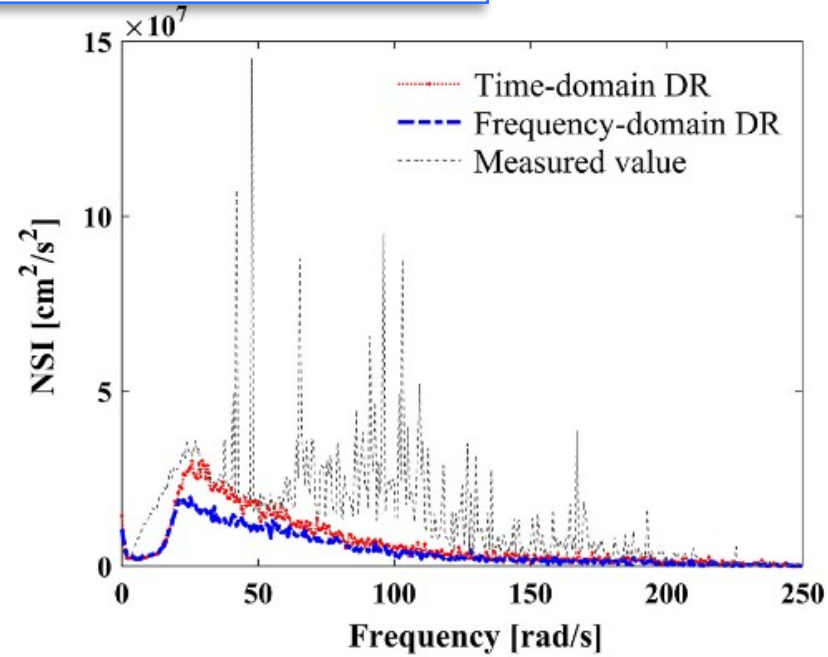
Non-stationarity index (NSI)

$$\text{NSI}(\omega) = \frac{1}{N} \sum_{i=0}^{N-1} \left( 1 - \frac{\Psi_{\bar{X}}(\omega, t_i)}{\psi_{\bar{X}}(\omega)} \right)^2$$

Hilbert spectrum

Boundary spectrum

523 sets of sample records for **stiff soils**



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# Conclusion

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## 1 Unified theoretical form

Through the derivation of the **time-domain representations** for stochastic processes, two **equivalent representations** of the stochastic processes in both frequency and time domains are clarified. Essentially, regardless of whether it is in the frequency or time domain, both stationary and non-stationary stochastic processes can be discretized into **a linear superposition form of a set of mutually uncorrelated random variables modulated by deterministic functions**.

## 2 Accuracy and efficiency

Compared with the conventional MCS, the DR simulation method offers **satisfactory accuracy**. At the meantime, the proposed trigonometric orthogonal basis achieves the comparable accuracy of MCS with a significantly reduced samples size, which not only **reduces the time needed** to generate stochastic process samples but also **enhances the efficiency** of nonlinear stochastic dynamic analysis for complex engineering structures.

## 3 Non-stationary characteristics

The NSI of the representative ground motion samples generated by the time-domain DR method **more closely resembles** that of real earthquake scenarios compared to the frequency-domain DR method, demonstrating that the time-domain DR method exhibits **superior capability in capturing non-stationary characteristics**.

## 4 Complete probabilistic set

By means of the deterministic point-selection methods for elementary random variables, all representative samples with assigned probabilities can thus constitute a **complete probability set**. As a result, the DR method provides a solid foundation to combine the **PDEM** for the accurate nonlinear stochastic dynamic response analysis and refined reliability assessment of complex engineering structures at full probability level.

# New Book Introduction: English Monograph



## Chapter 1 Fundamentals of Probability Theory and Stochastic Process

- §1.1 Probability and random variables
- §1.2 Functions of random variables
- §1.3 Fundamentals of Stochastic Process

## Chapter 2 Fundamentals of Earthquake Ground Motion

- §2.1 Amplitude characteristics of earthquake ground motion
- §2.2 Duration characteristics of earthquake ground motion
- §2.3 Spectral characteristics of earthquake ground motion
- §2.4 Non-stationary characteristics of earthquake ground motion
- §2.5 Spatial variability characteristics of earthquake ground motion

## Chapter 3 Dimension-reduction Representation of Univariate Stochastic Ground Motions:

### Frequency-domain Analysis Method

- §3.1 Original spectral representation of non-stationary stochastic processes
- §3.2 Conventional Monte Carlo methods
- §3.3 Novel dimension-reduction simulation methods
- §3.4 Numerical examples
- §3.5 Conclusion remarks

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Xinxin Ruan Ph.D.( 阮鑫鑫 博士 ), Xinyang Normal University( 信阳师范大学 )

Zhangjun Liu, Zixin Liu, Xinxin Ruan. Dimension-reduction Representation of Stochastic Ground Motion and Engineering Applications [M]. Springer Nature, 2026.

# New Book Introduction: English Monograph

## Chapter 4 Dimension-reduction Representation of Univariate Stochastic Ground Motion: Time-domain Analysis Method

- §4.1 Karhunen-Loève decomposition
- §4.2 Filtered white noise process models in physics
- §4.3 Time-domain representation of stochastic processes in mathematics
- §4.4 Novel dimension-reduction simulation methods
- §4.5 Numerical examples
- §4.6 Conclusion remarks

## Chapter 5: Dimension-reduction Representation of Multivariate Stochastic Ground Motions

- §5.1 Original spectral decomposition representation of non-stationary stochastic vector processes
- §5.2 Conventional Monte Carlo methods
- §5.3 Dimension-reduction simulation methods
- §5.4 Time-domain representation of non-stationary stochastic vector processes
- §5.5 Dimension-reduction representation of multi-point and multi-component stochastic ground motions

## Chapter 6: Dimension-reduction Representation of Stochastic Ground Motion fields

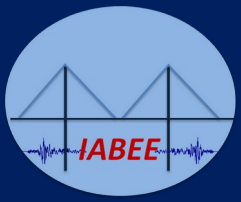
- §6.1 Original frequency-wavenumber spectral representation of spatio-temporal stochastic field
- §6.2 Conventional Monte Carlo methods of spatio-temporal stochastic field
- §6.3 Dimension-reduction methods of spatio-temporal stochastic field
- §6.4 Numerical examples of spatio-temporal stochastic field
- §6.5 Conclusion remarks

## Chapter 7: Dimension-reduction Modeling of Stochastic Ground Motion via Strong Motion Records

- §7.1 Statistical modeling of stochastic ground motion parameters in frequency domain
- §7.2 Statistical modeling of stochastic ground motion parameters in time domain
- §7.3 Dimension-reduction simulation of stochastic ground motion with random parameters
- §7.4 Conclusion remarks

## Chapter 8: Engineering Applications

- §8.1 Introduction to probability density evolution method
- §8.2 Random seismic response and reliability analysis of a nonlinear concrete gravity dam
- §8.3 Performance-based global reliability assessment of a high-rise frame-core tube structure
- §8.4 Global reliability evaluation of a high-pier long-span reinforced concrete rigid frame bridge
- §8.5 Conclusion remarks



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**Thanks for watching**  
**Questions are invited**

**Speaker: Xiaojiao Fu, Zhangjun Liu**

**Time: 2026.6.7**

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